www.EquipmentAnchorage.com

# PREMIER MOUNTS

AM65 WALL MOUNT

DES. J. ROBERSON

8/8/19

**JOB NO.** 11-1703

DATE

OF

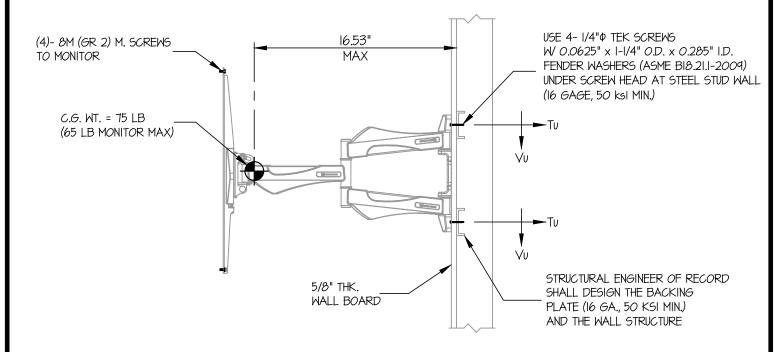
SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

WALL MOUNTED

No. 4197

SHEET



## STEEL STUD WALL SECTION (AM65 MOUNT)

#### NOTES:

1. FORCES ARE DETERMINED PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10.

STRENGTH DESIGN IS USED. (ap = 2.5, lp = 1.5, Rp = 2.5,  $\Omega_0$  = 2.0, z/h  $\leq$  1)

ANCHOR	MAX Sps	Ти мах	Ти мах
1/4"ø TEK SCREW	0.80	335	60
5/16"ø WOOD SCREW	1.00	396	73
5/16"ø (GR 5) BOLTS	2.20	756	152
1/4"ø HILTI HUS-EZ	0.70	600	121

- 2. CENTER OF GRAVITY (C.G.) AND WEIGHT ARE THE GOVERNING PARAMETERS FOR DESIGN. THIS PREAPPROVAL ENCOMPASSES ALL WEIGHTS UP TO THE MAXIMUM WEIGHT SHOWN.
- 3. STRUCTURAL ENGINEER OF RECORD FOR THE BUILDING SHALL PROVIDE SUPPORT STRUCTURE DESIGNED TO SUPPORT WEIGHTS AND FORCES SHOWN IN COMBINATION WITH ALL OTHER LOADS THAT MAY BE PRESENT.

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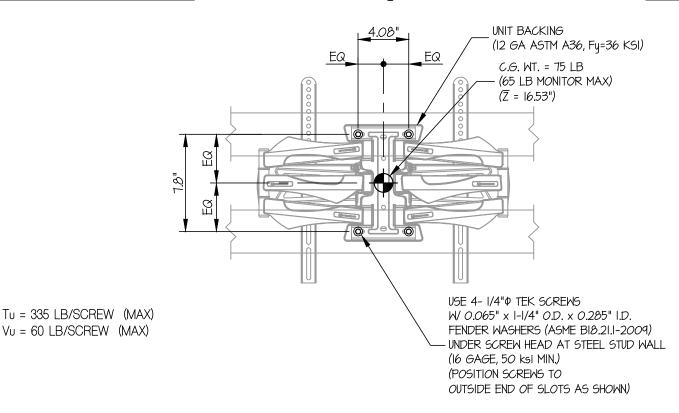
**SHEET 2** 

8 SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

MAX Sps < 0.80

**WALL MOUNTED** 



## ELEVATION AT STEEL STUD WALL (AM65 MOUNT)

LOADS: PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10. STRENGTH DESIGN IS USED (SDS = 0.80, 20,

WEIGHT = 75 LB

HORIZONTAL FORCE (E<sub>r</sub>) = 1.44 W<sub>p</sub> = 108 LB VERTICAL FORCE (E<sub>v</sub>) = 0.16 W<sub>p</sub> = 12 LB

**SCREW FORCES:** 

TENSION (T)

$$T_{u \text{ VERTICAL}} = \frac{(1.2(75\#) + 12\#)(16.53")}{2 \text{ SCREWS}(7.8")} = 108 \text{ LB/SCREW}$$

$$T_{u \text{ PARALLEL}} = \frac{108\#(16.53")}{2\text{sorews}(4.08")} = 219 \text{ LB/SCREW}$$

$$T_{u PERP.} = \frac{108\#}{4 \text{ screws}} = 27 \text{ LB/SCREW}$$

$$T_{u MAX} = 108# + (0.3)(27#) + 219# = 335 LB/SCREW (MAX)$$

SHEAR (V)

$$V_{u MAX} = \sqrt{\left(\frac{1.2(75\#) + 12\#}{4 \text{ screws}}\right)^2 + \left(\frac{108\#}{2 \text{ screws}}\right)^2} = 60 \text{ LB/SCREW (MAX)}$$

SCREW SPEC: 1/4"ø TEK SCREWS

 $\phi T = 412 LB/SCREW$  $\phi V = 362 LB/SCREW$ 

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.0$$

$$\left(\frac{335}{412}\right) + \left(\frac{60}{362}\right) = 0.99 \le 1.0 \quad \text{...} \quad \underline{\mathsf{O.K.}}$$

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## PREMIER MOUNTS

## AM65 WALL MOUNT

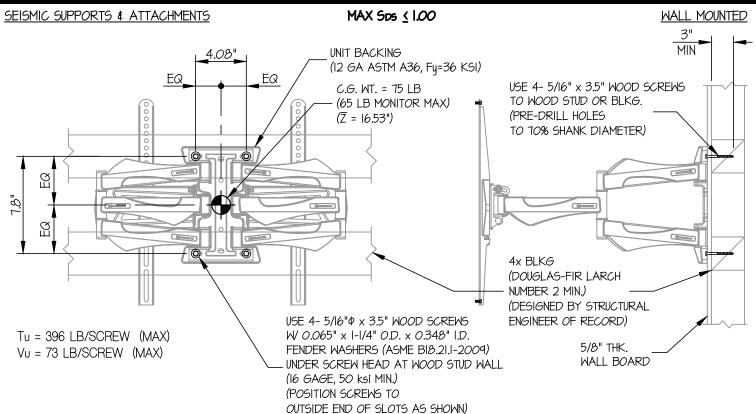
DES. J. ROBERSON

11-1703 JOB NO.

8/8/19 DATE

SHEET

SHEETS



## ELEVATION AT WOOD STUD WALL

(AM65 MOUNT)

WOOD STUD WALL SECTION (AM65 MOUNT)

LOADS: PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10. STRENGTH DESIGN IS USED (SDS = 1.00, Ap = 2.5, Ip = 1.5, Rp = 2.5,  $z/h \le 1$ ) WEIGHT = 75 LB

HORIZONTAL FORCE (Eh) = 1.80 Wp = 135 LB VERTICAL FORCE (E<sub>V</sub>) = 0.20 W<sub>p</sub> = 15 LB

SCREW FORCES:

TENSION (T)

$$T_{u \text{ VERTICAL}} = \frac{(1.2(75\#) + 15\#)(16.53")}{2 \text{ SCREWS}(7.8")} = 112 \text{ LB/SCREW}$$

$$T_{u \text{ PARALLEL}} = \frac{135\#(16.53")}{2\text{screws}(4.08")} = 274 \text{ LB/SCREW}$$

$$T_{u PERP.} = \frac{135\#}{4 \text{ screws}} = 34 \text{ LB/SCREW}$$

 $T_{IIMAX} = 112# + (0.3)(34#) + 274# = 396 LB/SCREW (MAX)$ 

SHEAR (V)

$$V_{u MAX} = \sqrt{\left(\frac{12(75\#) + 15\#}{4 \text{ screws}}\right)^2 + \left(\frac{135\#}{2 \text{ screws}}\right)^2} = 73 \text{ LB/SCREW (MAX)}$$

SCREW SPEC: 5/16" WOOD SCREWS

 $\phi T = 724 LB/SCREW$ 

 $\Phi V = 234 LB/SCREW$ 

 $\phi V = 216 LB/SCREW$  (DUE TO BENDING)

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 10$$

$$\left(\frac{396}{\mathsf{O}}\right) + \left(\frac{73}{\mathsf{O}}\right) = 089 < 10 \quad \text{and} \quad 0$$

$$\left(\frac{396}{724}\right) + \left(\frac{73}{216}\right) = 0.89 \le 1.0$$
 .°. O.K.

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## AM65 WALL MOUNT

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SHEET

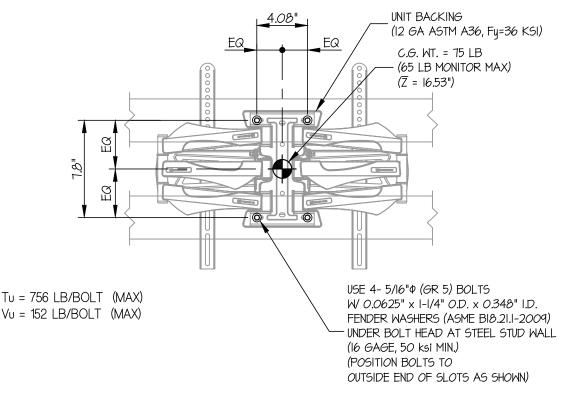
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8 SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

MAX Sps < 2.20

<u>WALL MOUNTED</u>



## ELEVATION AT STEEL STUD WALL (AM65 MOUNT)

LOADS: PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10. STRENGTH DESIGN IS USED (SDS = 2.20,  $\Delta p$  = 2.5, |p| = 1.5, Rp = 2.5,  $z/h \le 1$ )

WEIGHT = 75 LB

HORIZONTAL FORCE (En) = 3.96 Wp = 297 LB

VERTICAL FORCE (Ev) = 0.44 Wp = 33 LB

**BOLT FORCES:** 

TENSION (T)

$$T_{u \text{ VERTICAL}} = \frac{(1.2(75\#) + 33\#)(16.53")}{2 \text{ BOLTS}} = 131 \text{ LB/BOLT}$$

$$T_{u \text{ PARALLEL}} = \frac{297 \# (16.53'')}{2 \text{Bolts} (4.08'')} = 602 \text{ LB/BOLT}$$

$$T_{\text{U PERP.}} = \frac{297\#}{4 \text{ BOLTS}} = 74 \text{ LB/BOLT}$$

$$T_{\text{LLMAX}} = 131\# + (0.3)(74\#) + 602\# = 756 \text{ LB/BOLT (MAX)}$$

SHEAR (V)

$$V_{u MAX} = \sqrt{\left(\frac{1.2(75\#) + 33\#}{4 \text{ BOLTS}}\right)^2 + \left(\frac{297\#}{2 \text{ BOLTS}}\right)^2} = 152 \text{ LB/BOLT (MAX)}$$

BOLT SPEC: 5/16"ø (GR 5) BOLTS

 $\phi T = 5177 LB/BOLT$  $\phi V = 2698 LB/BOLT$ 

$$\left(\frac{T_u}{\Phi T}\right) + \left(\frac{V_u}{\Phi V}\right) \le 1.0$$

$$\left(\frac{756}{5177}\right) + \left(\frac{152}{2698}\right) = 0.21 \le 1.0 \quad ... \quad \underline{O.K.}$$

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## PREMIER MOUNTS

AM65 WALL MOUNT

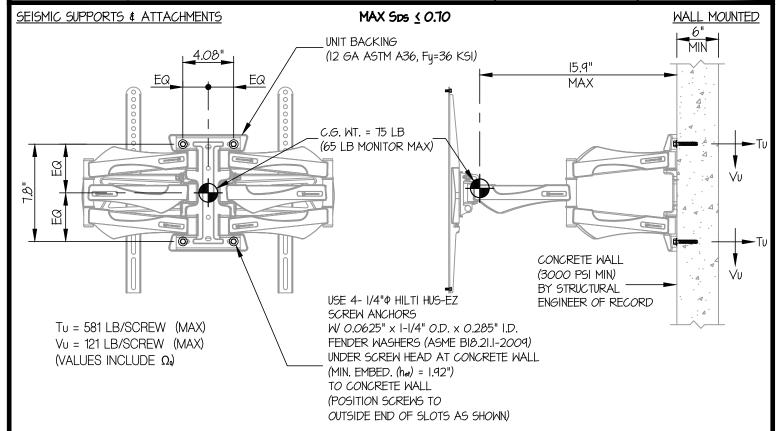
DES. J. ROBERSON

JOB NO. 11-1703

DATE 8/8/19

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**8** SHEETS



#### ELEVATION AT CONCRETE WALL

(AM65 MOUNT)

CONCRETE WALL SECTION
(AM65 MOUNT)

LOADS: PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10.

STRENGTH DESIGN IS USED (SDS = 0.70,  $\Delta_p$  = 2.5,  $I_p$  = 1.5,  $R_p$  = 2.5,  $\Omega_o$  = 2.0, z/h < 1)

WEIGHT = 75 LB

HORIZONTAL FORCE (Eh) = 1.26 Wp = 95 LB

HORIZONTAL FORCE (Emh) = 3.15 Wp = 236 LB

VERTICAL FORCE (E<sub>v</sub>) = 0.14 W<sub>p</sub> = 11 LB

SCREW FORCES:

TENSION (T)

$$T_{u \text{ VERTICAL}} = \frac{(1.2(75\#) + 11\#)(15.9")}{2 \text{ screws}(7.8")} = 103 \text{ LB/SCREW}$$

$$T_{u \text{ PARALLEL}} = \frac{236\#(15.9")}{2\text{SCREWS}(4.08")} = 460 \text{ LB/SCREW}$$

$$T_{\text{u PERP.}} = \frac{236\#}{4 \text{ screws}} = 59 \text{ LB/SCREW}$$

 $T_{UMAX} = 103# + (0.3)(59#) + 460# = 581 LB/SCREW (MAX)$ 

SHEAR (V)

$$V_{u \text{ MAX}} = \sqrt{\left(\frac{1.2(75\#) + 11\#}{4 \text{ screws}}\right)^2 + \left(\frac{236\#}{4 \text{ screws}}\right)^2} = 121 \text{ LB/SCREW (MAX)}$$

BOLT SPECS: 1/4" HILTI HUS-EZ \$\phi\$T = 0.75 \$\phi\$Nn = 623 LB/SCREW (TENSION) \$\phi\$V = \$\phi\$Vn = 836 LB/SCREW (SHEAR)

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 12$$

$$\left(\frac{581}{623}\right) + \left(\frac{121}{836}\right) = 108 \le 12 \quad \text{°} \quad \underline{\mathsf{O.K.}}$$

# EQUIPMENT ANCHORAGE & SEISMIC ENGINEERING www.EquipmentAnchorage.com

# PREMIER MOUNTS AM95 WALL MOUNT

DES. J. ROBERSON

8/8/19

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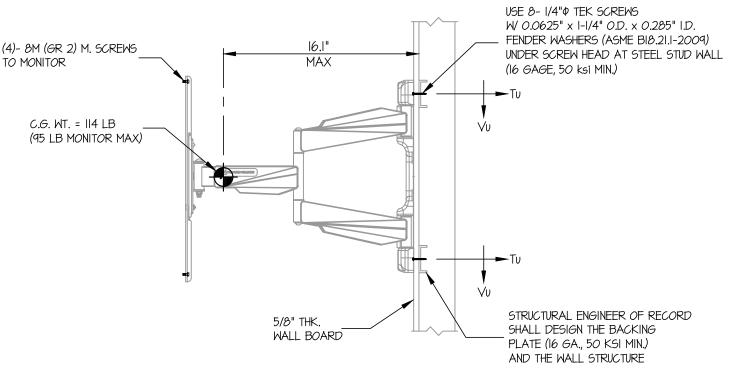
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F 8 SHEETS

SHEET

SEISMIC SUPPORTS & ATTACHMENTS

<u>WALL MOUNTED</u>



## STEEL STUD WALL SECTION (AM95 MOUNT)

#### NOTES:

1. FORCES ARE DETERMINED PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10

STRENGTH DESIGN IS USED. (Sps = 2.20, 2p = 2.5, 2p = 1.5, 2p = 2.5, 2p = 2.5, 2p = 2.0, 2p

HORIZONTAL FORCE (En) = 3.96 Wp HORIZONTAL FORCE (Emh) = 7.92 Wp (FOR CONCRETE ANCHORAGE) VERTICAL FORCE (Ev) = 0.44 Wp

- 2. CENTER OF GRAVITY (C.G.) AND WEIGHT ARE THE GOVERNING PARAMETERS FOR DESIGN. THIS PREAPPROVAL ENCOMPASSES ALL WEIGHTS UP TO THE MAXIMUM WEIGHT SHOWN.
- 3. STRUCTURAL ENGINEER OF RECORD FOR THE BUILDING SHALL PROVIDE SUPPORT STRUCTURE DESIGNED TO SUPPORT WEIGHTS AND FORCES SHOWN IN COMBINATION WITH ALL OTHER LOADS THAT MAY BE PRESENT.

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## PREMIER MOUNTS

## AM95 WALL MOUNT

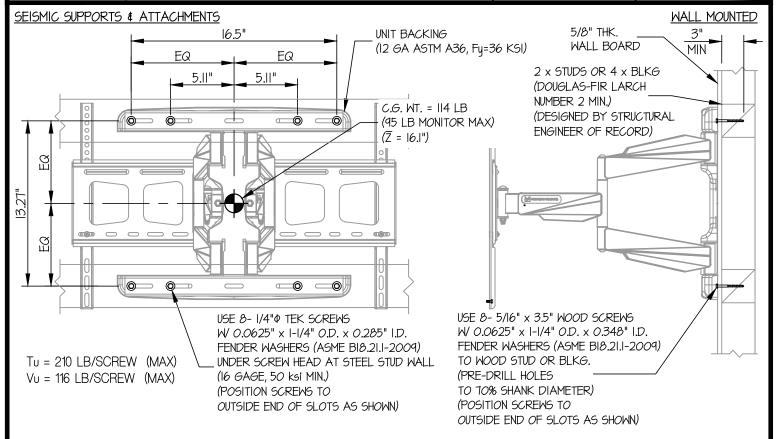
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SHEET

SHEETS



#### ELEVATION AT STEEL STUD WALL (AM95 MOUNT)

LOADS: PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10. STRENGTH DESIGN IS USED (Sps = 2.20, 2p = 2.5, lp = 1.5, Rp = 2.5, z/h < 1)

WEIGHT = 114 LB HORIZONTAL FORCE (En) = 3,96 Wp = 452 LB VERTICAL FORCE (E<sub>v</sub>) = 0.44 W<sub>p</sub> = 50 LB SCREW FORCES:

TENSION (T)

$$T_{u \text{ VERTICAL}} = \frac{(1.2(114\#) + 50\#)(16.1")}{4 \text{screws} (13.27")} = 57 \text{ LB/SCREW}$$

$$T_{u \text{ PARALLEL}} = \frac{452 \# (16.1'')}{4 \text{ SCREWS} (13.36'')} = 136 \text{ LB/SCREW}$$

$$T_{\text{U PERP.}} = \frac{452\#}{8 \text{ screws}} = 57 \text{ LB/SCREW}$$

$$T_{\text{IIMAX}} = 57\# + (0.3)(57\#) + 136\# = 210 \text{ LB/SCREW (MAX)}$$

SHEAR (V)

$$V_{u MAX} = \sqrt{\left(\frac{12(114\#) + 50\#}{8 \text{ SCREWS}}\right)^2 + \left(\frac{452\#}{4 \text{ SCREWS}}\right)^2} = 116 LB/SCREW (MAX)$$

SCREW SPEC: 1/4"ø TEK SCREWS

ΦT = 418 LB/SCREW φV = 362 LB/SCREW

UNITY CHECK:

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.0$$

$$\left(\frac{210}{418}\right) + \left(\frac{116}{362}\right) = 0.83 \le 1.0 \quad \text{...} \quad \underline{\mathsf{O.K.}}$$

WOOD STUD WALL SECTION

(AM95 MOUNT)

SCREW SPEC: 5/16" Ø WOOD SCREWS

 $\Phi T = 724 LB/SCREW$ 

ΦV = 234 LB/SCREW

 $\phi V = 216 LB/SCREW$  (DUE TO BENDING)

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.0$$

$$\left(\frac{210}{657}\right) + \left(\frac{116}{212}\right) = 0.87 \le 10 \quad \text{...} \quad \underline{\mathsf{O.K.}}$$

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## PREMIER MOUNTS

## AM95 WALL MOUNT

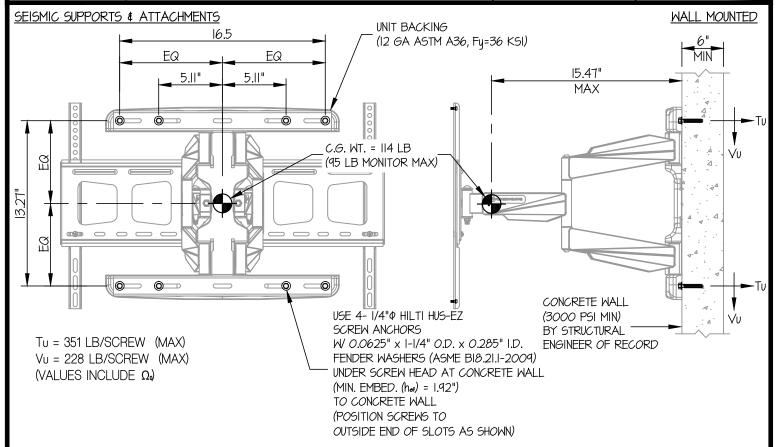
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8/8/19 DATE

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SHEETS



#### ELEVATION AT CONCRETE WALL

(AM95 MOUNT)

CONCRETE WALL SECTION (AM95 MOUNT)

LOADS: PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10. STRENGTH DESIGN IS USED (SDS = 2.20,  $\Delta p$  = 2.5, lp = 1.5, Rp = 2.5,  $z/h \le 1$ )

HORIZONTAL FORCE (Eh) = 3.96 Wp = 452 LB

HORIZONTAL FORCE (Emh) = 7.92 Wp = 904 LB

VERTICAL FORCE (E<sub>V</sub>) = 0.44 W<sub>p</sub> = 50 LB

SCREW FORCES:

TENSION (T)

$$T_{u \text{ VERTICAL}} = \frac{(1.2(114\#) + 50\#)(15.47")}{4 \text{ SCREWS}} = 55 \text{ LB/SCREW}$$

$$T_{u \text{ PARALLEL}} = \frac{904\#(15.47")}{4 \text{ screws}(13.36")} = 262 \text{ LB/SCREW}$$

$$T_{\text{U PERP.}} = \frac{904\#}{8 \text{ screws}} = 113 \text{ LB/SCREW}$$

 $T_{IJ,MAX} = 55# + (0.3)(113#) + 262# = 351 LB/SCREW (MAX)$ 

SHEAR (V)

$$V_{u \text{ MAX}} = \sqrt{\left(\frac{1.2(114\#) + 50\#}{8 \text{ screws}}\right)^2 + \left(\frac{904\#}{4 \text{ screws}}\right)^2} = 228 \text{ LB/SCREW (MAX)}$$

BOLT SPECS: 1/4" HILTI HUS-EZ  $\phi T = 0.75 \phi Nn = 623 LB/SCREW (TENSION)$  $\phi V = \phi V n = 836 LB/SCREW$  (SHEAR)

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.2$$

$$\left(\frac{351}{623}\right) + \left(\frac{228}{836}\right) = 0.84 \le 1.2 \quad \text{°} \quad \underline{\mathsf{O.K.}}$$